GENERAL REQUIREMENTS

1. THE CONTRACTOR SHALL PERFORM ALL WORK IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (IBC) 2018
2. WHERE A SECTION OR DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL APPLY TO ALL SIMILAR CONDITIONS.
3. THE DESIGN ADEQUACY AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
4. DO NOT SCALE DRAWINGS, FOLLOW DIMENSIONS SHOWN ON PLANS.
5. CONTRACTOR SHALL COORDINATE ALL DIMENSIONS AND ELEVATIONS SHOWN IN, SECTIONS & DETAILS PRIOR TO CONSTRUCTION OR MATERIAL PURCHASED AND SHALL NOTIFY ARCHITECT OR ENGINEER IN WRITING OF DISCREPANCIES.

FOUNDATION NOTES

1. FOUNDATION DESIGNED FOR 1,500 PSF MAXIMUM ALLOWABLE BEARING PRESSURE BASED ON GEOTECHNICAL REPORT NO. B-093.20 PREPARED BY CSRA TESTING & ENGINEERING COMPANY, INC., AND DATED OCTOBER 21, 2020.
2. ALL FOUNDATION FILL SUBGRADE SOILS SHALL BE COMPACTED AS FOLLOWS.
   (REF. ASTM D-698)
   A. 95% STANDARD PROCTOR FOR GREATER THAN 18" BELOW FINAL FILL.
   B. 98% STANDARD PROCTOR FOR THE UPPER 18" BENEATH BUILDINGS AND PAVEMENTS.
3. SOILS TESTING LABORATORY SHALL CONDUCT COMPACTED TESTS IN ACCORDANCE WITH ASTM D-698. RATE OF COMPACTED SHALL BE AS FOLLOWS.
   A. ONE TEST FOR EACH 50 LINEAR FEET OF CONTINUOUS FOOTING.
   B. ONE TEST FOR EACH 1000 S.F. OF SLAB.
4. REMOVE ALL WATER SOFTEN SOILS FROM FOOTING EXCAVATIONS PRIOR TO PLACING CONCRETE. FILL REMAINING VOIDS WITH ADDITIONAL CONCRETE.
5. SUPPORT ALL BOTTOM REINFORCEMENT IN FOUNDATION WITH CONCRETE BRICKS AT 48" O.C. MAX.
6. ALL FOOTING, AND OTHER FOUNDATION REINFORCING SHALL BE TIED IN PLACE PRIOR TO POURING CONCRETE.
7. WHERE FINISH GRADES DIFFER ON OPPOSITE SIDES OF FOUNDATION WALLS, PROVIDE TEMPORARY BRACING, PREVENT LATERAL MOVEMENT UNTIL ALL ADJACENT FILLING, COMPACTION, FLOOR SLABS, AND FRAMING AT NEXT LEVEL OVER HAS BEEN COMPLETED
9. WHERE GRAVITY PLUMBING LINES OCCUR BELOW TOP OF WALL FOOTING, STEP FOOTING DOWN TO PROVIDE CLEARANCE. INTERFERENCE OFFSET AT GRAVITY SEWER UNLESS OTHERWISE SPECIFIED. COORDINATE WITH PLUMBING DRAWINGS FOR LOCATIONS, SIZES, AND INVERTS.
10. CONSTRUCTION JOINTS IN CONTINUOUS FOOTING TO BE FORMED VERTICALLY IN ACCORDANCE WITH FOUNDATION DETAILS IN PLANS.

CAST IN PLACE CONCRETE

1. THE FOLLOWING ACI STANDARDS (LATEST EDITION) APPLY:
   A. ACI 318 - CODE
   B. ACI 315 - DETAILING
   C. ACI 301 - SPECIFICATIONS
   D. ACI 304 - PLACING
   E. ACI 347 - FORMWORK
   F. ACI 211.1 - MIX PROPORTIONING
   G. ACI 305 - HOT WEATHER CONCRETING
   H. ACI 306 - COLD WEATHER CONCRETING

2. ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE (145 PCF) WITH MIXES DESIGN THE FOLLOWING CRITERIA

STRUCTURAL ELEMENTS	28-DAY COMPRESSIVE STRENGTH
SLAB-ON-GRADE	3,000 PSI
FOOTINGS	3,000 PSI

SLAB ON GRADE

1. APPLY AN APPROVED CURING COMPOUND CONFORMING TO ASTM C 309 AFTER FINISHING THE SLAB.
2. ALL WELDED WIRE FABRIC SHALL BE IN ACCORDANCE WITH ASTM A1064. LAP ADJOINING PIECES AT LEAST ONE FULL MESH OR 8" MINIMUM, UNLESS OTHERWISE APPROVED. ALL WELDED WIRE FABRIC SHALL BE BLOCKED INTO POSITION INDICATED WITH PRECAST CONCRETE BLOCKS HAVING A COMPRESSIVE STRENGTH EQUAL TO THAT OF THE SLAB.
3. THE MAXIMUM SPACING OF JOINTS SHALL BE 15' OR AS SHOWN ON PLANS.
4. ALL PURPOSE FILL MATERIAL SHALL BE CLEAN GRANULAR MATERIAL WITH 100% PASSING 1-1/2" SIEVE AND NO MORE THAN 5% PASSING A NO. 4 SIEVE. POROUS FILL SHALL BE COMPACTED TO 95% MAX. DRY DENSITY PER ASTM D-698.
5. SLAB JOINTS SHALL BE FILLED WITH APPROVED MATERIAL. THIS SHOULD TAKE PLACE AS LATE AS POSSIBLE, PREFERABLY 4 TO 6 WEEKS AFTER THE SLAB HAS BEEN CAST. PRIOR TO FILLING, REMOVE ALL DEBRIS FROM THE SLAB JOINTS, THEN FILL IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS.
6. SEE THE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS OF DEPRESSED SLAB AREAS AND DRAINS. SLOPE SLAB TO DRAWINGS WHERE SHOWN.
7. THE FINISH TOLERANCE OF ALL SLABS SHALL BE IN ACCORDANCE WITH ACI 301, TYPE-A
8. WALKWAYS AND OTHER EXTERIOR ARE NOT INDICATED ON THE STRUCTURAL DRAWINGS. SEE THE SITE PLAN AND ARCHITECTURAL DRAWINGS FOR LOCATIONS, DIMENSIONS, ELEVATIONS, JOINTING DETAIL, AND FINISH DETAILS.
9. CONTROL JOINT SEALANT SHALL BE SIKADUR 51 NS/SL OR APPROVED EQUAL. EXPANSION AND ISOLATION JOINT SEALANT SHALL BE SIKAFLEX 1a OR APPROVED EQUAL.
10. MINIMUM REINFORCING STEEL CLEAR COVERS ARE AS FOLLOWS:
    A. CONCRETE CAST IN PLACE DIRECTLY AGAINST EARTH...3"
    B. SLAB ON GRADE...3/4"
11. TIMING OF THE SAWCUT:
    A. WET CONVENTIONAL SAW CUT PROCESS SHOULD BE 4 TO 12 HOURS AFTER SLAB PLACEMENT; 4 HOURS IN HOT WEATHER AND 12 HOURS IN COLD WEATHER.
    B. EARLY-ENTRY DRY CUT PROCESS SHOULD BE 1 TO R HOURS AFTER SLAB PLACEMENT; 1 HOUR IN HOT WEATHER AND 4 HOURS IN COLD WEATHER.
12. DEPTH OF THE SAWCUT:
    A. WET CONVENTIONAL SAWCUT PROCESS: 1 INCH.
    B. EARLY-ENTRY DRY-CUT PROCESS: 1-1/2 INCHES.
13. SLAB ON GRADE WWR (WELDED WIRE REINFORCEMENT) SPECIFIED IN PLAN CAN BE SUBSTITUTED WITH STRUCTURAL MACROSYNTHETIC FIBRILLATED POLYPROPYLENE FIBER AS APPROVED BY THE ENGINEER OF RECORD. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO PRODUCE THE CONVERSION OF THE WWR TO EQUIVALENT AMOUNT OF STRUCTURAL FIBER. THE CALCULATION TO CONVERT THE WWR TO EQUIVALENT STRUCTURAL FIBER SHALL BEAR THE SEAL OF A PROFESSIONAL ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS BEING CONSTRUCTED AND FORWARDED TO THE ENGINEER OF RECORD FOR REVIEW AT LEAST FIVE (5) DAY S PRIOR TO THE DAY THAT IT IS TO BE UTILIZED IN CONSTRUCTION

SUBGRADE PREPARATION

1. CONTRACTOR SHALL STRIP AND REMOVE ALL VEGETATION TOPSOIL, ROOTS, AND ORGANIC SOILS FROM THE CONSTRUCTION AREA FOR A DISTANCE OF AT LEAST 10' BEYOND THE EXTENT OF BUILDING FOUNDATION LIMITS. THE DEPTH OF STRIPPING SHALL BE THAT REQUIRED TO REMOVE SIGNIFICANT ROOT ZONES, SMALL TREE STUMPS, AND OTHER UNACCEPTABLE MATERIALS, BUT IN NO CASE SHALL IT BE LESS THAN 12".
2. PROOFROLLING SHALL BE PERFORMED UNDER THE OBSERVATION OF AN APPROVED TESTING LABORATORY SUPERVISED BY A GEOTECHNICAL ENGINEER. UNDERCUT, BACKFILL, AND COMPACT AREAS WHICH PUMP, DEFLECT, OR RUT EXCESSIVELY OR WHICH DO NOT STABILIZE AFTER SUCCESSIVE PASSES OF PROOFROLLING EQUIPMENT.

DIMENSIONAL LUMBER FRAMING

1. ALL STRUCTURAL LUMBER DESIGN SHALL CONFORM TO THE REQUIREMENTS OF THE NATIONAL DESIGNS SPECIFICATION FOR WOOD CONSTRUCTION (NDS-2018).
2. LOCATION, NUMBER, AND DIMENSIONS OF FRAMING MEMBERS SHOW GENERAL ARRANGEMENT ONLY. ACTUAL SPANS, SPACINGS, ETC., SHALL BE DETERMINED FROM ARCHITECTURAL DETAILS.
3. SEE ARCHITECTURAL PLANS AND DETAILS FOR EDGE SECTIONS, HEADER AND LINTEL LOCATIONS, AND ALL NON-STRUCTURAL FRAMING AND TRIM.
4. ALL WOOD FRAMING MATERIAL SHALL BE SURFACED DRY AND USED AT 19% MAXIMUM MOISTURE CONTENT. ALLOWABLE STRESS REQUIREMENTS OF ALL MATERIALS SHALL BE IN ACCORDANCE WITH THE "SCHEDULE OF REQUIRED STRESS VALUES."
5. ALL STUD AND WALL FRAMING SHALL BE NO. 2 GRADE SOUTHERN YELLOW PINE (SYP) OR SPRUCE PINE. THE "STUD" GRADE MATERIAL IS STRICTLY PROHIBITED FROM USE.
6. ALL JOIST, RAFTER & MISC. FRAMING SHALL BE NO. 1 GRADE, SOUTHERN YELLOW PINE. PROVIDE FULL-DEPTH BLOCKING AT ENDS. PROVIDE FULL-DEPTH (OR METAL) BRIDGING AT MIDSPAN AND AT A MAXIMUM SPACING OF 8'-0" O/C IN BETWEEN.
7. ALL LUMBER EXPOSED TO EXTERIOR ENVIRONMENT OR IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED TO A MINIMUM RETENTION OF 0.25 lbs. OF ACQ PER CUBIC FOOT OF WOOD, AND EACH PIECE SHALL BEAR THE THIRD PARTY QUALITY MARK, "ABOVE GRADE USE." ALL LUMBER IN CONTACT WITH THE GROUND SHALL BE PRESSURE TREATED TO A MINIMUM RETENTION OF 0.40 lbs. OF ACQ PER CUBIC FOOT OF WOOD, AND EACH PIECE SHALL BEAR THE THIRD PARTY QUALITY MARK, "GROUND CONTACT USE". REFERENCE STANDARD AWPA C2 AND ASTM D1760 FOR PRESSURE TREATMENT OF TIMBER PRODUCTS.
8. FOR LUMBER EXPOSED TO WEATHER AND WHERE POSSIBLE, ALL CUTS AND HOLES SHOULD BE COMPLETED BEFORE TREATMENT. CUTS AND HOLES DUE TO ON-SITE FABRICATION SHALL BE BRUSHED WITH 2 COATS OF COPPER NAPHTHENATE SOLUTION CONTAINING A MINIMUM OF 2% METALLIC COPPER IN SOLUTION (PER AWPA STD. M4.)
9. THE CONTRACTOR SHALL CAREFULLY SELECT LUMBER TO BE USED IN LOAD BEARING APPLICATIONS. THE LENGTH OF SPLIT ON THE WIDE FACE OF 2" NOMINAL LOAD BEARING FRAMING SHALL BE LIMITED TO LESS THAN ⅓ OF THE WIDE FACE DIMENSION. THE LENGTH OF SPLIT ON THE WIDE FACE OF 3" (NOMINAL) AND THICKER LUMBER SHALL BE LIMITED TO ½ OF THE NARROW FACE DIMENSIONS.
10. ALL WOOD JOISTS AND HEADERS WHICH FRAME INTO BEAMS SHALL BE SUPPORTED BY SIMPSON LUS HANGERS WITH THE SAME WIDTH AND DEPTH OF THE MEMBER, U.N.O. USE HANGERS WITH CONCEALED FLANGES WHERE THE CONNECTOR CANNOT BE HIDDEN BY WOOD TRIM OR THE SUPPORT MEMBER IS WIDER THAN THE STANDARD FLANGE.
11. PROVIDE NAILING PATTERN IN COMPLIANCE WITH IBC RECOMMENDED FASTENING SCHEDULE.
12. LOAD BEARING STUD WALLS SHALL BE CONTINUOUSLY BRIDGED AT MID-HEIGHT AND UNSUPPORTED PLYWOOD WALL SHEATHING JOINTS WITH SOLID WOOD BLOCKING, U.N.O.
13. NO CUTS, HOLES, OR COPES IN STRUCTURAL WOOD FRAMING SHALL BE PERMITTED WITHOUT PRIOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER AND ARCHITECT.
14. PROVIDE FULL-DEPTH BLOCKING AT ENDS OF RAFTERS AND JOISTS. PROVIDE FULL-DEPTH (OR METAL) BRIDGING AT MIDSPAN AND AT A MAXIMUM SPACING OF 8'-0" O.C. IN BETWEEN.
15. STRUCTURAL STEEL PLATE CONNECTORS SHALL CONFORM TO ASTM A 36 SPECIFICATIONS AND BE ¼" THICK UNLESS OTHERWISE INDICATED. BOLTS CONNECTING WOOD MEMBERS SHALL BE PER ASTM A 307 AND BE 1/2" DIAMETER UNLESS OTHERWISE INDICATED. PROVIDE WASHERS FOR ALL BOLT HEADS AND NUTS IN CONTACT WITH WOOD SURFACES.
16. BOLT HOLES SHALL BE CAREFULLY CENTERED AND DRILLED NOT MORE THAN 1/16" LARGER THAN THE BOLT DIAMETER. BOLTED CONNECTIONS SHALL BE SNUGGED TIGHT BUT NOT TO THE EXTENT OF CRUSHING WOOD UNDER WASHERS.
17. HOLES AND NOTCHES DRILLED OR CUT INTO WOOD FRAMING SHALL NOT EXCEED THE REQUIREMENTS OF IBC, SECTION 23.
18. ALL PLATES, ANCHORS, NAILS, BOLTS, NUTS, WASHERS, AND OTHER MISCELLANEOUS HARDWARE SHALL BE HOT DIP GALVANIZED.
19. ADEQUATE BRACING SHALL BE PROVIDED UNTIL PERMANENT BRACING AND/OR DIAPHRAGMS ARE INSTALLED.
20. OVERLAP ALL TOP PLATES AT CORNERS.
21. JOINTS IN THE DOUBLE TOP PLATES OF WALLS SHALL BE OFFSET A MINIMUM LAP LENGTH OF 4'-0" & NAILED WITH NOT LESS THAN (15) 16d FACE NAILS ON EACH SIDE OF THE JOINT.
22. ALL BEAMS SHALL HAVE LATERAL SUPPORT AT THE COMPRESSION EDGE AT A MAXIMUM OF 24" O.C. AND AT BEARING POINTS, WITH A MINIMUM BEARING LENGTH OF 3 1/2."
23. PROVIDE DOUBLE JOISTS OR SOLID BLOCKING AT 24" O.C. UNDER ALL PARTITIONS AND TO SUPPORT CONCENTRATED LOADS FROM FRAMING ABOVE, UNLESS NOTED OTHERWISE.
24. PROVIDE DOUBLE HEADER BEAMS OF THE SAME SIZE AS JOISTS OR RAFTERS TO FRAME AROUND OPENINGS IN PLYWOOD DECK UNLESS OTHERWISE INDICATED.
25. PROVIDE HEADERS, BRIDGING, CONNECTORS, BLOCKING, TRIMMERS, ETC. AS REQUIRED AND RECOMMENDED BY AITC TIMBER CONSTRUCTION STANDARDS AND IBC 2018 U.N.O.
26. WOOD FRAMING MATERIALS:
    A. ALL DIMENSIONAL LUMBER (EXCEPT WALL STUDS) SHALL BE #2 SYP KD OR BETTER AND PROVIDE NOT LESS THAN THE DESIGN VALUES LISTED IN NDS 2018 SUPPLEMENT.
    B. PREFABRICATED "MICRO-LAM" LUMBER HEADERS AND BEAMS SHALL BE AS MANUFACTURED BY "TRUSS JOIST WEYERHAEUSER", OR APPROVED EQUAL. DO NOT CUT OR NOTCH MICRO-LAM MATERIAL WITHOUT THE MANUFACTURER'S APPROVAL.
    C. PRE-ENGINEERED MEMBERS SHALL HAVE THE FOLLOWING PROPERTIES:
       LVL Fb = 2600 psi Fv = 285 psi E = 2.0 mpsi Fc = 750 psi
       PARALAM PSL Fb = 2809 psi Fv = 290 psi E = 2.0 mpsi Fc = 625 psi
    D. SHEATHING:
       WALLS.....APA RATED SHEATHING, EXPOSURE 1 OR EXTERIOR
       ROOF.....APA RATED SHEATHING, EXPOSURE1, 2, OR EXTERIOR
       FLOOR.....APA RATED STURD-I-FLOOR
    E. ALL BOLTS SHALL BE ASTM A307 WITH WASHERS, GALVANIZED.
    F. NAILS IN ACCORDANCE WITH MINIMUM NAILING REQUIREMENTS OF IBC EXCEPT WHERE NOTED IN DETAILS OR SPECIFICATIONS. ALL NAILS TO BE GALVANIZED.

REQUIRED LAP SPLICE LENGTH FOR 3,000 PSI CONCRETE	
BAR SIZE	LAP LENGTH
#4	30"
#5	36"

PLYWOOD/GYP BOARD SHEATHING

1. ALL PLYWOOD CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE AMERICAN PLYWOOD ASSOCIATION (APA) SPECIFICATIONS.
2. ALL ROOF PANEL SHEATHING SHALL BE PER PLAN. SUITABLE EDGE SUPPORT SHALL BE PROVIDED BY THE USE OF PANEL CLIPS OR BLOCKING BETWEEN FRAMING.
3. ALL FLOOR SHEATHING SHALL BE TONGUE & GROOVE PER PANEL. FIELD-GLUE USING ADHESIVES MEETING APA SPECIFICATIONS AFG-01, APPLIED IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.
4. ALL PANEL SHEATHING SHALL BE PER PLAN.
5. INSTALL ALL PLYWOOD SHEATHING AT FLOOR AND ROOF WITH THE LONG DIMENSION OF THE PANEL ACROSS SUPPORTS AND WITH PANEL CONTINUOUS OVER TWO OR MORE SPANS. STAGGER PANEL END JOINTS. ALLOW 1/8" SPACING AT PAN EL ENDS AND EDGES UNLESS OTHERWISE RECOMMENDED BY THE SHEATHING MANUFACTURER.
6. ALL NAILS SHALL NOT BE OVERDRIVEN.
7. PROVIDE BLOCKING AT UNSUPPORTED PANEL EDGES AS FOLLOWS:
   ROOFS AND FLOORS - ONLY WHERE INDICATED ON PLAN
   WALLS - FULLY BLOCKED
8. WHERE EITHER 2" OR 2 1/2" FASTENER SPACINGS ARE USED FOR WOOD STRUCTURAL PANELS USED AT ROOF OR FLOOR, THE FRAMING MEMBER AT THE ADJOINING PANEL SHALL BE 3" NOMINAL WIDTH AND THE NAILS AT PANEL EDGES SHALL BE STAGGERED IN TWO LINES.
9. ALL PLYWOOD AND SHEATHING SHALL BE APA RATED, BEAR THE STAMP OF AN APPROVED TESTING AGENCY, AND SHALL BE FABRICATED WITH EXTERIOR GLUE. NAILS AT ABUTTING PLYWOOD EDGES MUST PENETRATE THE SAME PIECE OF FRAMING OR BLOCKING.
   OPENINGS IN SHEATHING (DIAPHRAGMS):
   A. PERIMETER OF ANY OPENINGS SHALL BE FASTENED AS BOUNDARY NAILING.
   B. OPENINGS WITH A DIMENSION PERPENDICULAR TO THE JOISTS GREATER THAN 4'-0" SHALL BE BLOCKED BEYOND THE HEADERS, AND METAL TIES NOT LESS THAN 16 GAGE BY 1-1/2" WIDE WITH 8-16d COMMON NAILS ON EACH SIDE OF THE HEADER-JOIST INTERSECTION SHALL BE PROVIDED.
   C. AT SHEAR WALLS OR DRAG STRUTS, THE PLYWOOD DIAPHRAGM MUST BE NAILED TO TOP PLATES, MEMBERS, BLOCKING, ETC. AS FOLLOWS:
      EXTERIOR WALLS/ DIAPHRAGM EDGES: 1x BOUNDARY NAILING
      INTERIOR WALL LINES: 2x BOUNDARY NAILING

PRE ENGINEERED WOOD TRUSSES

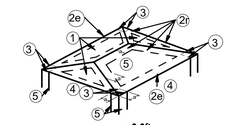
1. WOOD TRUSS SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH THE APPLICABLE PROVISIONS OF THE LATEST EDITION OF THE NATIONAL DESIGN SPECIFICATIONS OF THE NATIONAL FOREST PRODUCTS ASSOCIATION. THE DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES OF THE TRUSS PLATE INSTITUTE AND INTERNATINAL BUILDING CODE.
2. ALL TRUSSED RAFTERS SHALL BE DESIGNED, FABRICATED, AND ERECTED TO SUPPORT THE FOLLOWING MINIMUM LOADS:
   LIVE - TOP CHORD = 20 PSF
   DEAD - TOP CHORD = 20 PSF
   DEAD - BOTTOM CHORD = 20 PSF + MECHANICAL EQUIPMENT
   WIND - TOP CHORD GROSS UPLIFT = 30 PSF
3. UNLESS NOTED OTHERWISE, WOOD MATERIALS SHALL BE SOUTHERN PINE, DOUGLAS FIR OR LARCH AND SHALL BE KILN DRIED AND USED AT 19% MAXIMUM MOISTURE CONTENT. PROVIDE GRADE NO. 2 OR AS REQUIRED TO SATISFY STRESS REQUIREMENTS.
4. UNLESS NOTED OTHERWISE, PROVIDE AT EACH SUPPORT GALVANIZED METAL HURRICANE PLATE ANCHORAGES SUFFICIENT TO RESIST HORIZONTAL AND VERTICAL WIND COMPONENTS WITH A SAFETY FACTOR OF 3. MINIMUM UPLIFT LOAD AS DETERMINED BY TRUSS MANUFACTURER FROM REQUIREMENTS OF LOCAL BUILDING CODES.
5. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY AND PERMANENT BRACING AS REQUIRED FOR SAFE ERECTION AND PERFORMANCE OF THE TRUSSES. THE GUIDELINES SET RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" SHALL BE A MINIMUM REQUIREMENT.
6. SHOP DRAWINGS, COMPUTATIONS, ETC. TO BE SUBMITTED FOR REVIEW. SHOP DRAWINGS SHALL PROVIDE ERECTION LAYOUT FOR TRUSS RAFTERS, OUTRIGGERS, HEADERS,BRACING,ETC. SEE ARCHITECTURAL AND STRUCTURAL PLANS FOR SUPPORT LOCATIONS.
7. CALCULATIONS AND DRAWINGS SHALL BEAR THE SEAL OF A PROFFESSIONAL ENGINEER LICENSED IN THE STATE IN WHICH THE PROFECT IS BEING CONSTRUCTED.
8. TRUSS WEB TO CHORD CONNECTION SHALL BE MADE WITH APPROVED GALVANIZED STEEL TRUSS CONNECTION PLATES MEETING ALL REQUIREMENTS OF THE TRUSS PLATE INSTITUTE.
9. TRUSS MEMBERS AND COMPONENTS SHALL NOT BE CUT, NOTCHED, DRILLED NOR OTHERWISE ALTERED IN ANY WAY WITHOUT WRITTEN APPROVAL OF THE ENGINEER.

DESIGN CRITERIA

1. BUILDING CODE:
   2018 INTERNATIONAL BUILDING CODE
2. GRAVITATIONAL LOADS (ASCE 7-16):

	CONCENTRATED	DISTRIBUTED
ROOF		
	LL = N/A	LL = 20 PSF
	DL = N/A	DL = 20 PSF
GROSS UPLIFT = 30 PSF		
COLLATERAL LOAD		CL = 5 PSF
1ST FLOOR		
	LL = N/A	LL = 40 PSF
	DL = 2,000 LBS	DL = 15 PSF

3. WIND LOADS (ASCE 7-16):
   BASIC WIND SPEED (3 SEC GUST) = 112 MPH
   RISK CATEGORY = II
   EXPOSURE CATEGORY = B
   GC_{pi}= ± 0.18
4. IBC 2018 / ASCE 7-16
   RISK CATEGORY = II
   IMPORTANCE FACTOR = 1.00
   DESIGN CATEGORY = C
   SITE CLASS = D
   S_s = 0.226 g
   S_1 = 0.097 g
   S_{DS} = 0.241 g S_{D1} = 0.155 g
   SEISMIC FORCE RESISTING SYSTEM = LIGHT-FRAME (WOOD) WALLS SHEATHED WITH WOOD STRUCTURAL PANELS.
   SEISMIC RESPONSE COEFFICIENT, C_s = 0.0371
   RESPONSE MODIFICATION FACTOR, R = 6.5
   ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE
   SEISMIC BASE SHEAR = C_sW WHERE W=WEIGHT OF STRUCTURE



a= 6.6ft

COMPONENTS AND CLADDING WIND PRESSURES (PSF)

	ZONE	AREA (SF)	+P	-P
ROOF	1	10	21.8	-39.9
		20	19.3	-33.9
		50	16.1	-25.8
		100	13.7	-19.8
	2e	10	21.8	-39.9
		20	19.3	-33.9
		50	16.1	-25.8
		100	13.7	-19.8
	2n	10	21.8	-44.0
		20	19.3	-39.3
		50	16.1	-33.1
		100	13.7	-28.5
	2r	10	21.8	-39.9
		20	19.3	-33.9
		50	16.1	-25.8
		100	13.7	-19.8
	3e	10	21.8	-53.9
		20	19.3	-47.8
		50	16.1	-39.7
		100	13.7	-33.5
	3r	10	21.8	-44.0
		20	19.3	-39.3
		50	16.1	-33.1
		100	13.7	-28.5
WALLS	4	10	23.8	-25.8
		20	22.7	-24.7
		50	21.3	-23.3
		100	20.2	-22.2
	5	10	23.8	-31.9
		20	22.7	-29.7
		50	21.3	-26.9
		100	20.2	-24.7

WIND SPEED = 115 MPH
ROOF SLOPE= 27° TO 45°

PRYCO STRUCTURAL
PROFESSIONAL ENGINEERS & CONSULTANTS
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PROJECT TITLE:
JUD C HICKEY CENTER

REVISIONS			
REV #	DATE	APRVD BY	REVISION
	04-16-21	JRH	35% SET
	11-17-21	JRH	65% SET
	01-20-22	JRH	95% SET
	02-16-22	JRH	ISSUE FOR PERMIT
1	09-09-22	JRH	OWNER REVISIONS

GEORGIA REGISTERED No. PE036027 PROFESSIONAL ENGINEER JOSE R. HERNANDEZ
09/09/2022

DA PROJECT NUMBER & NAME:
1893

SHEET TITLE:
STRUCTURAL NOTES

SHEET NO:
S100

REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION PER IBC 2018 CH. 17

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD	IBC REFERENCE
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	–	X	ACI 318: CH 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2.REINFORCING BAR WELDING:				
A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706:	–	X		
B. INSPECT SINGLE -PASS FILLET WELDS, MAXIMUM 5/16"; AND	–	X	AWS D1.4 ACI 318: 26.6.4	
C. INSPECT ALL OTHER WELDS	–	–		
3. INSPECT ANCHORS CAST IN CONCRETE	–	X	ACI 318: 17.8.23	
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MAMBERS. A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS. B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.A.	–	X	ACI 318: 17.8.24 ACI 318: 17.8.2	
5. VERIFY USE OF REQUIRED DESIGN MIX.	–	X	ACI 318: CH 19,26.4.3, 26.4.4	1904.1,1904.2, 1908.2, 1908.3
6. PRIOR TO CONRETE PLACEMENT FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPEREATURE OF TEH CONCRETE.	X	–	ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	1908.1
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	–	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8. VERIFY MAINTANANCE OF SPECIFIED CURING TEMPEREATURE AND TECHNIQUES.	–	X	ACI 318: 26.5.3-26.5.5	1908.9
9. INSPECT PRESTRESSED CONCRETE FOR.				
A. APPLICATION OF PRESTRESSED FORCES: AND	–	–	ACI 318: 26.10	
B. GROUTING OF BONDED PRESTRESSING TENDONS.	–	–		
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	–	–	ACI 318: 26.9	
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	–	–	ACI 318: 26.11.2	
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEBER BEING FORMED.	–	X	ACI 318: 26.11.1.2(B)	

REQUIRED SPECIAL INSPECTION AND TESTS OF SOILS PER IBC 2018 CH. 17

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. VERIFY MATERIALS BELOW SHOLLOW FOUNMDATION ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	–	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	–	X
3. PEFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	–	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	X	–
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	–	X

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PROJECT TITLE:

JUD C HICKEY CENTER

REVISIONS

REV #	DATE	APRVD BY	REVISION
	04-16-21	JRH	35% SET
	11-17-21	JRH	65% SET
	02-16-22	JRH	ISSUE FOR PERMIT
1	09-09-22	JRH	OWNER REVISIONS

GEORGIA REGISTERED No. PE036027 PROFESSIONAL ENGINEER JOSE R. HERNANDEZ
09/09/2022

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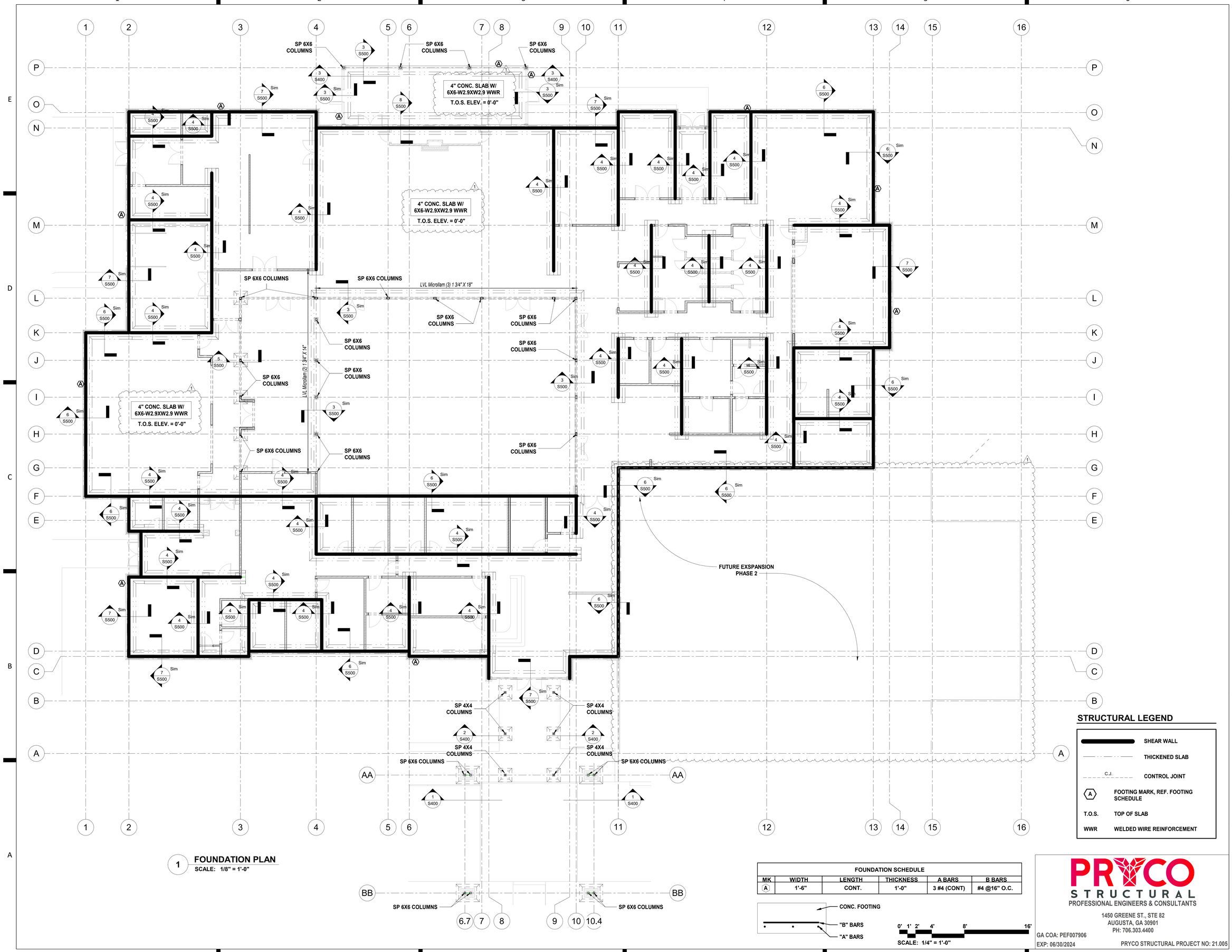
GA COA: PEF007906
EXP: 06/30/2024
PRYCO STRUCTURAL PROJECT NO: 21.005

DA PROJECT NUMBER & NAME:
1893

SHEET TITLE:
SPECIAL INSPECTIONS

SHEET NO:
S110



1 **FOUNDATION PLAN**
SCALE: 1/8" = 1'-0"

STRUCTURAL LEGEND

	SHEAR WALL
	THICKENED SLAB
C.J.	CONTROL JOINT
A	FOOTING MARK, REF. FOOTING SCHEDULE
T.O.S.	TOP OF SLAB
WWR	WELDED WIRE REINFORCEMENT

FOUNDATION SCHEDULE

MK	WIDTH	LENGTH	THICKNESS	A BARS	B BARS
A	1'-6"	CONT.	1'-0"	3 #4 (CONT)	#4 @16" O.C.

0' 1' 2' 4' 8' 16'
SCALE: 1/4" = 1'-0"

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PROJECT TITLE:
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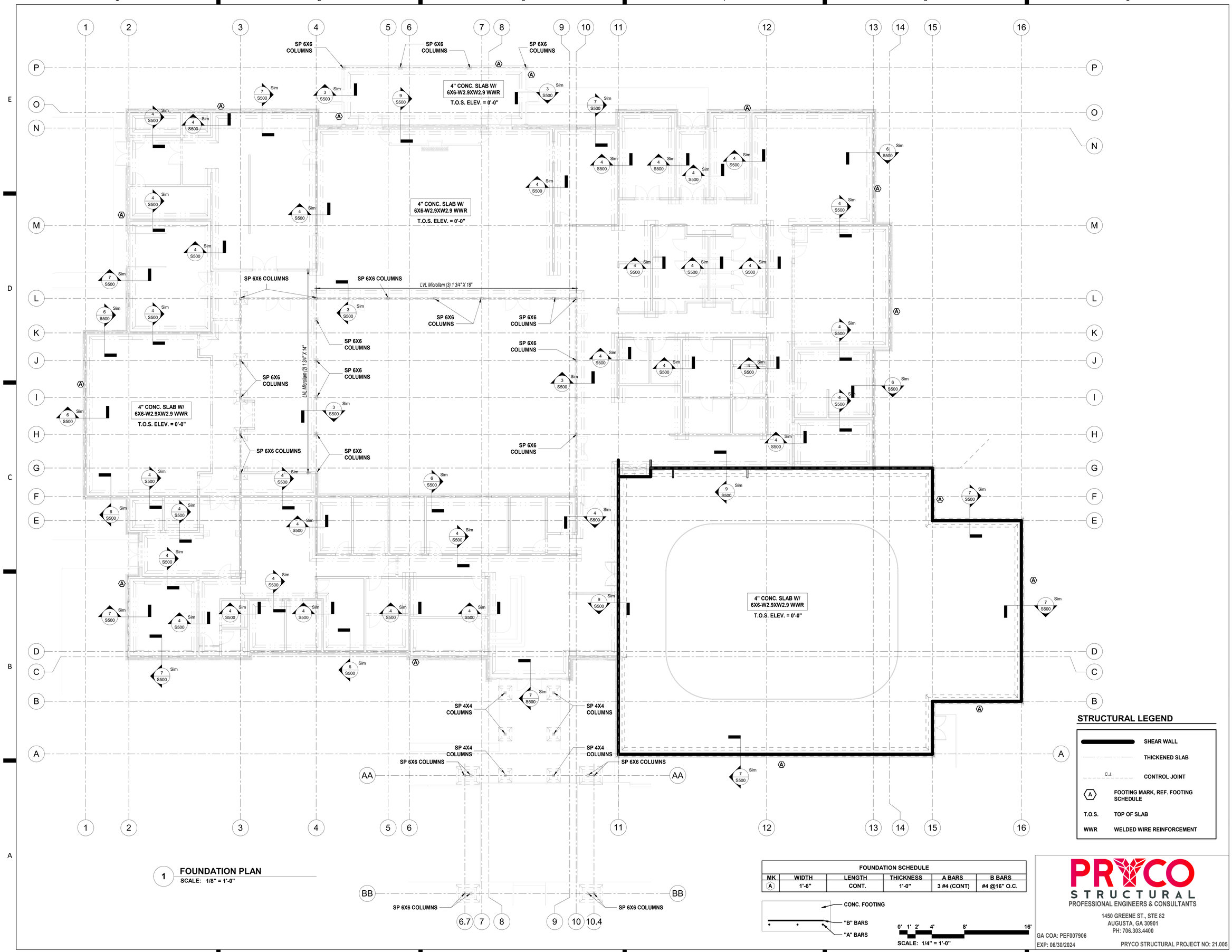
	02-16-22	JRH	ISSUE FOR PERMIT
1	09-09-22	JRH	OWNER REVISIONS

GEORGIA REGISTERED No. PE036027 PROFESSIONAL ENGINEER JOSE R. HERNANDEZ
09/09/2022

DA PROJECT NUMBER & NAME:
1893

SHEET TITLE:
FOUNDATION PLAN

SHEET NO:
S200



1 FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

FOUNDATION SCHEDULE					
MK	WIDTH	LENGTH	THICKNESS	A BARS	B BARS
A	1'-6"	CONT.	1'-0"	3 #4 (CONT)	#4 @16" O.C.

0' 1' 2' 4' 8' 16'

SCALE: 1/4" = 1'-0"

STRUCTURAL LEGEND

	SHEAR WALL
	THICKENED SLAB
C.J.	CONTROL JOINT
A	FOOTING MARK, REF. FOOTING SCHEDULE
T.O.S.	TOP OF SLAB
WWR	WELDED WIRE REINFORCEMENT

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PRYCO STRUCTURAL PROJECT NO: 21.005

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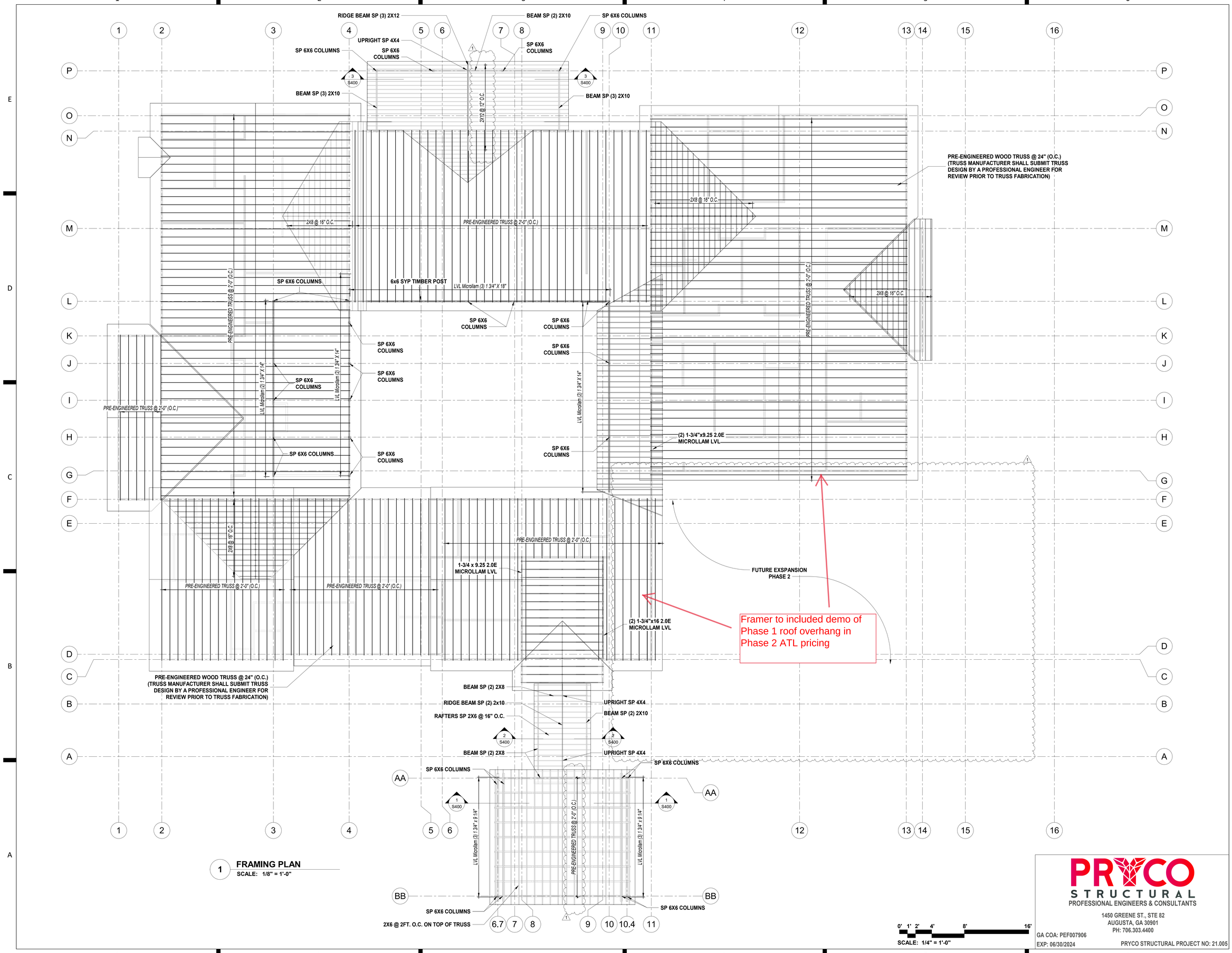
GEORGIA REGISTERED PROFESSIONAL ENGINEER No. PE036027 JOSE R. HERNANDEZ
09/09/2022

DA PROJECT NUMBER & NAME:
1893

SHEET TITLE:
FOUNDATION PLAN PHASE 2

SHEET NO:
S201



1 **FRAMING PLAN**
SCALE: 1/8" = 1'-0"

0' 1' 2' 4' 8' 16'
SCALE: 1/4" = 1'-0"

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1	09-09-22	JRH	OWNER REVISIONS

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GA COA: PEF007906
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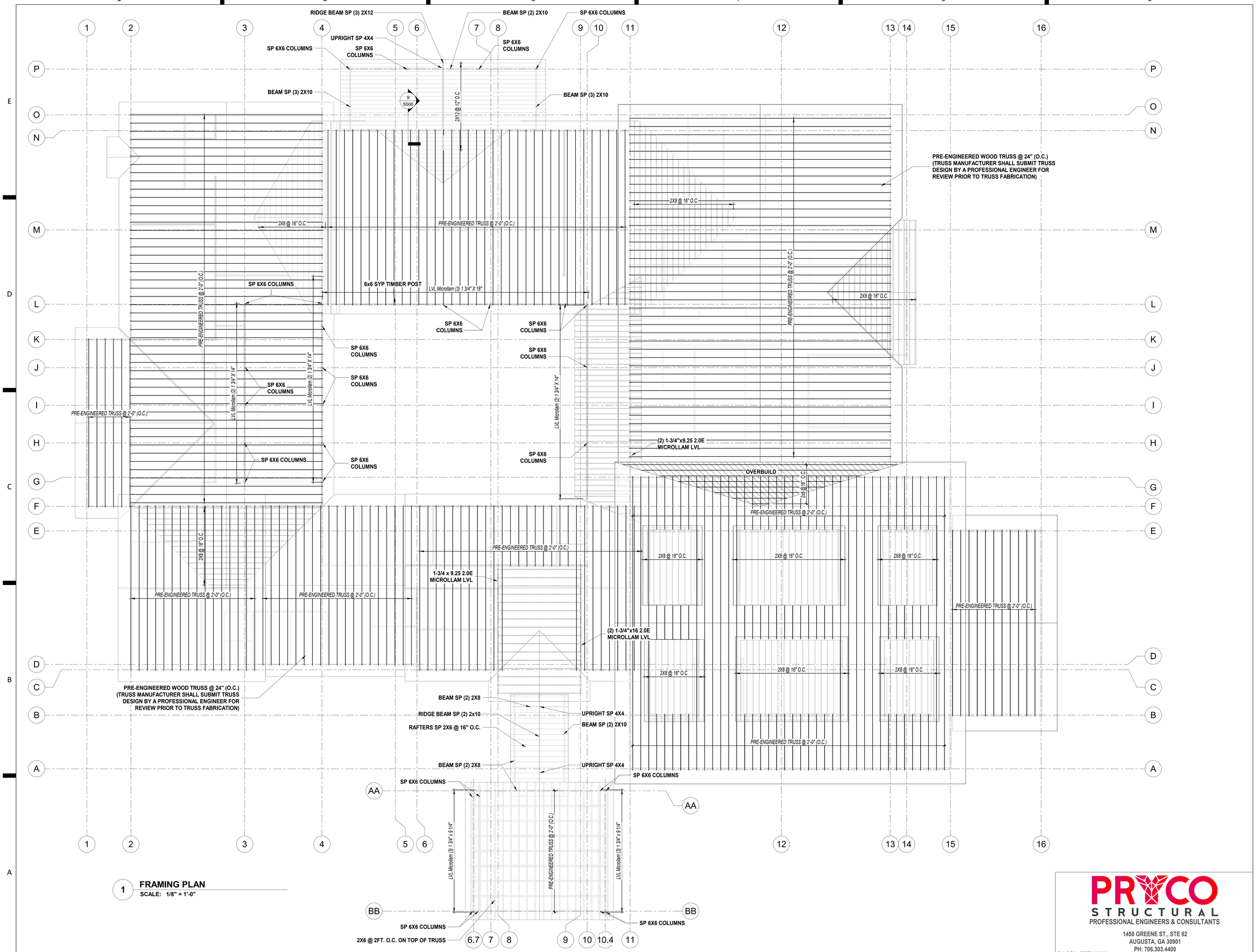
PRYCO STRUCTURAL PROJECT NO: 21.005

DA PROJECT NUMBER & NAME:
1893

SHEET TITLE:
FRAMING PLAN

SHEET NO:
S300



1 **FRAMING PLAN**
SCALE: 1/8" = 1'-0"

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	01-20-22	JRH	95% SET
	02-16-22	JRH	ISSUE FOR PERMIT
1	09-09-22	JRH	OWNER REVISIONS

GEORGIA REGISTERED No. PE036027 PROFESSIONAL ENGINEER JOSEPH R. HERNANDEZ
09/09/2022

DA PROJECT NUMBER & NAME:
1893

SHEET TITLE:
FRAMING PLAN PHASE 2

SHEET NO:
S301

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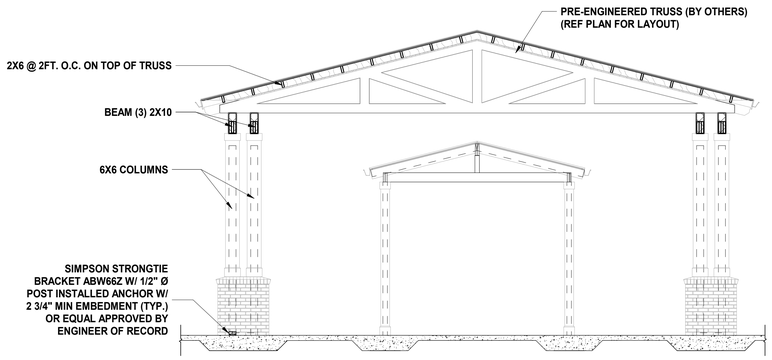
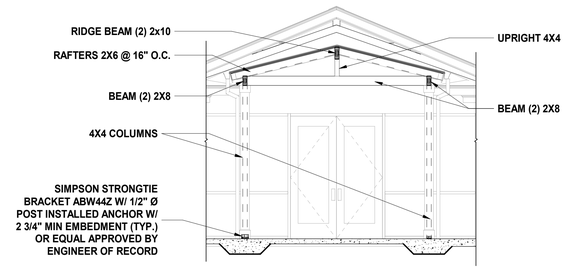
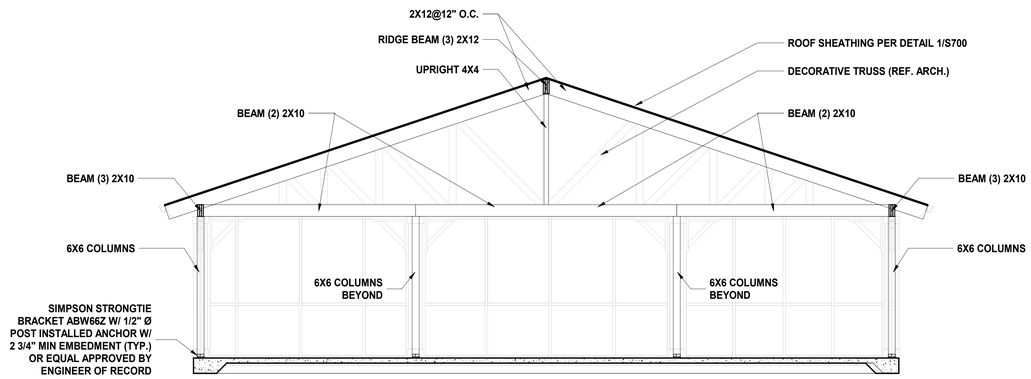
PRYCO STRUCTURAL PROJECT NO: 21.005



1 / S200 **BUILDING SECTION**
SCALE: 1/4" = 1'-0"



2 / S200 **BUILDING SECTION**
SCALE: 1/4" = 1'-0"



3 / S200 **BUILDING SECTION**
SCALE: 1/4" = 1'-0"

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REV #	DATE	APRVD BY	REVISION
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	01-20-22	JRH	95% SET
	02-16-22	JRH	ISSUE FOR PERMIT
1	09-09-22	JRH	OWNER REVISIONS

GEORGIA REGISTERED No. PE036027 PROFESSIONAL ENGINEER JOSEPH R. HERNANDEZ
09/09/2022

DA PROJECT NUMBER & NAME:
1893

SHEET TITLE:
BUILDING
SECTION

SHEET NO:
S400

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GA COA: PEF007906
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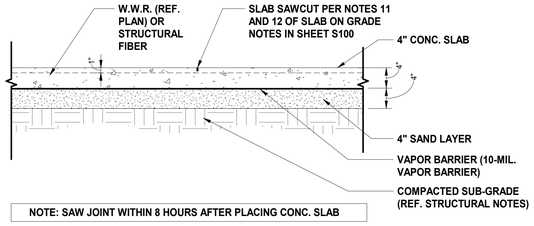
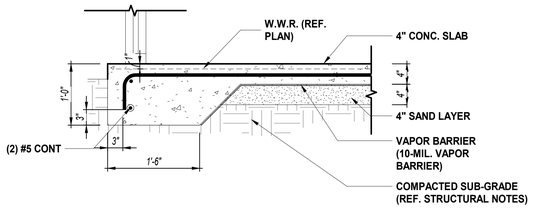
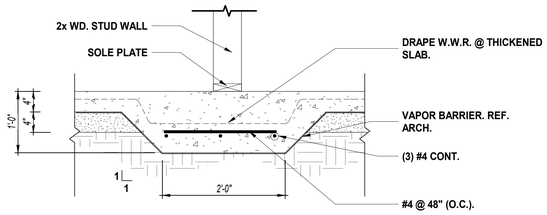
PRYCO STRUCTURAL PROJECT NO: 21.005



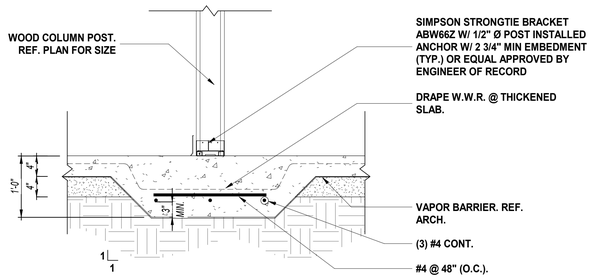
1 CONCRETE SLAB ON GRADE 4"
S200 SCALE: 1" = 1'-0"



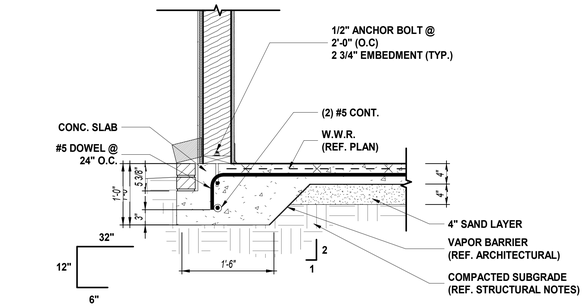
3 TURNDOWN SLAB DETAIL
S200 SCALE: 1" = 1'-0"



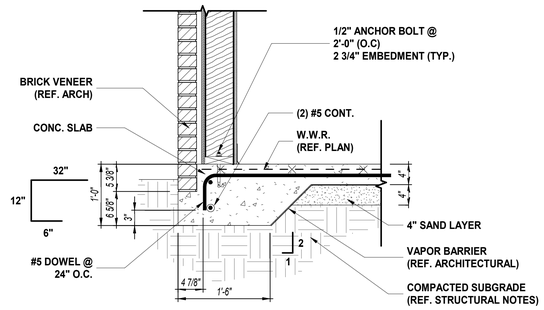
4 THICKENED SLAB AT WALL DETAIL
S200 SCALE: 1" = 1'-0"



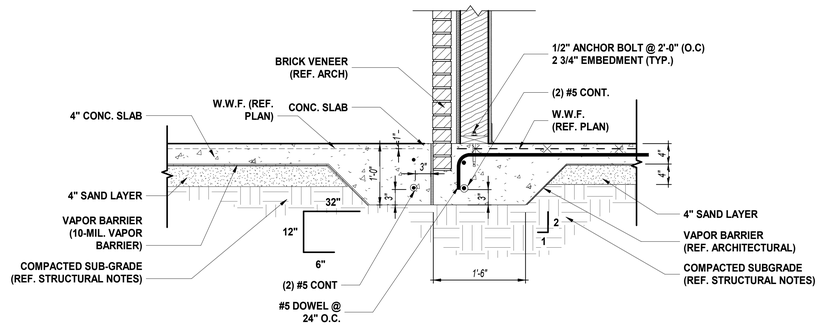
5 THICKENED SLAB AT COLUMN
S200 SCALE: 1" = 1'-0"



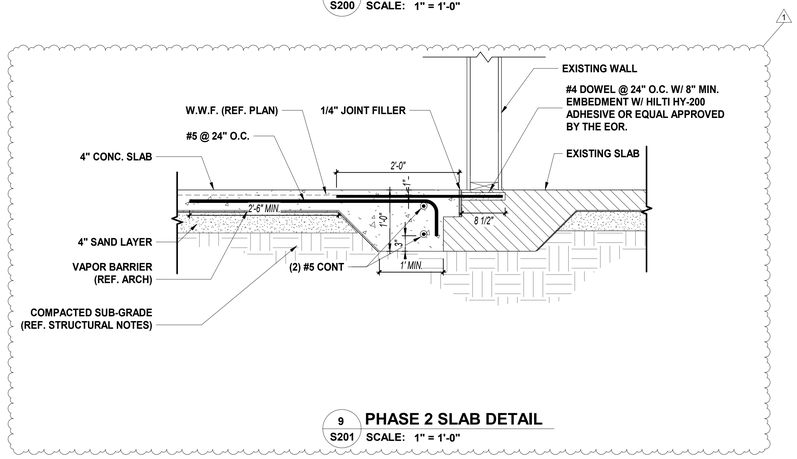
6 TURNDOWN SLAB DETAIL
S200 SCALE: 1" = 1'-0"



7 TURNDOWN SLAB DETAIL
S200 SCALE: 1" = 1'-0"



8 TURNDOWN SLAB DETAIL
S200 SCALE: 1" = 1'-0"



9 PHASE 2 SLAB DETAIL
S201 SCALE: 1" = 1'-0"

0' 1' 2' 4' 8' 16'
SCALE: 1/4" = 1'-0"
0' 6" 1' 2' 4' 8'
SCALE: 1/2" = 1'-0"
0' 4" 8" 1'-4" 2'-8" 4'
SCALE: 3/4" = 1'-0"

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PROJECT TITLE:
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REVISIONS

REV #	DATE	APRVD BY	REVISION
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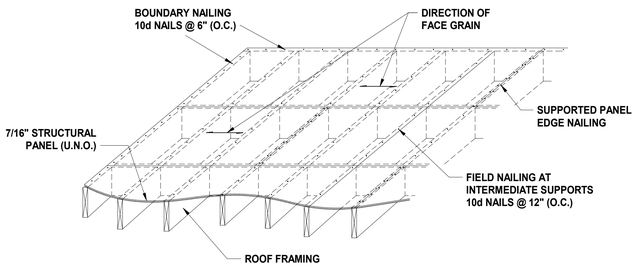
GEORGIA REGISTERED PROFESSIONAL ENGINEER No. PE036027 JOSE R. HERNANDEZ
09/09/2022

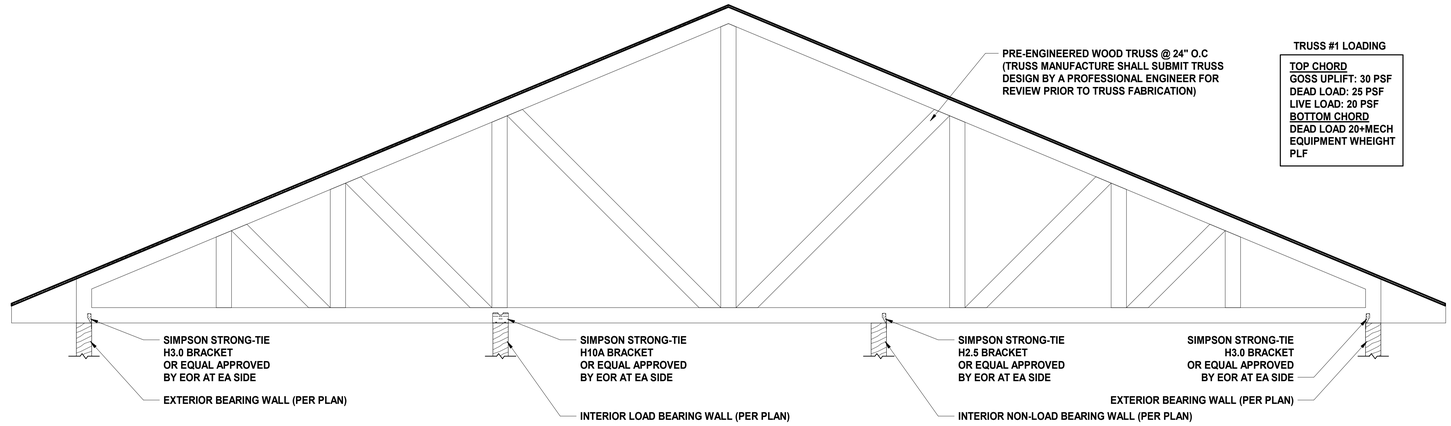
DA PROJECT NUMBER & NAME:
1893

SHEET TITLE:
FOUNDATION DETAILS

SHEET NO:
S500



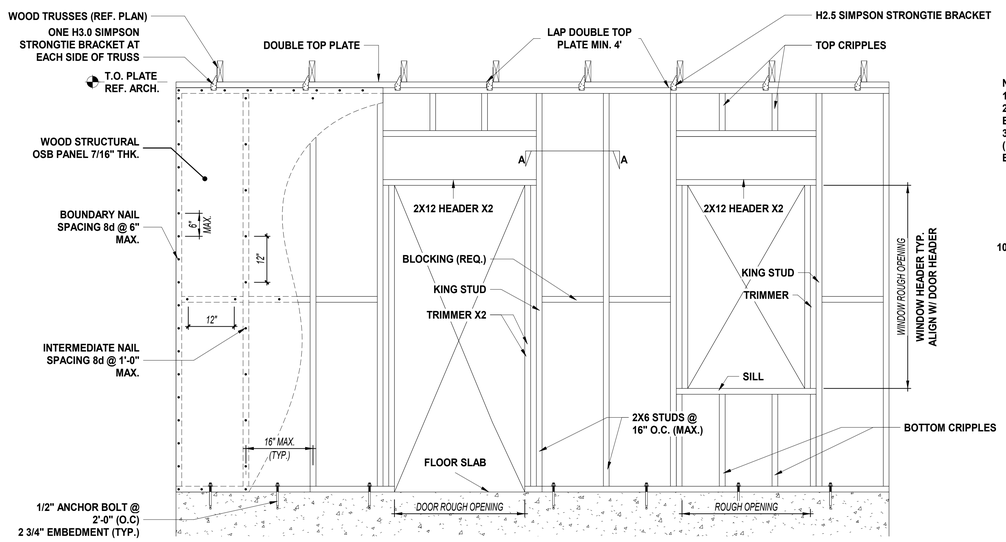
1 ROOF SHEATHING
SCALE: 1" = 1'-0"



TRUSS #1 LOADING

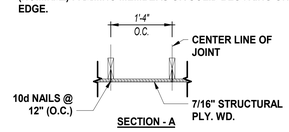
TOP CHORD
GOSS UPLIFT: 30 PSF
DEAD LOAD: 25 PSF
LIVE LOAD: 20 PSF
BOTTOM CHORD
DEAD LOAD 20+MECH
EQUIPMENT WHEIGHT
PLF

3 TYPICAL PRE-ENGINEERED TRUSS CONNCETIONS
SCALE: 1/2" = 1'-0"



2 SHEAR WALL DETAIL
SCALE: 3/4" = 1'-0"

NOTE:
1. ALL JOINTS SHALL BE SOLID BLOCKED
2. PERIMETER OF WINDOW/DOOR ROUGH OPENINGS SHALL BE FASTENED AS BOUNDARY NAILING.
3. ALL PANEL EDGES SHALL BE BACKED WITH 2" OR WIDER (NOMINAL) FRAMING MEMBERS OR SOLID BLOCKING ON EDGE.



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GEORGIA REGISTERED No. PE036007 PROFESSIONAL ENGINEER JOSEPH R. HERNANDEZ
09/09/2022

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DA PROJECT NUMBER & NAME:
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SHEET TITLE:
FRAMING
DETAILS

SHEET NO:
S700